



**SR EN 1993-1-6/NA**

**STANDARD ROMÂN**

..... 2011

**Eurocode 3: Design of steel structures.  
Part 1-6: Strength and Stability of Shell  
Structures  
National Annex**

Eurocod 3: Proiectarea structurilor de oțel  
Partea 1-6: Rezistența și stabilitatea plăcilor curbe subțiri  
Anexa națională

Eurocode 3. Calcul des structures en acier.  
Partie 1-6: Résistance et stabilité des structures en coque  
Annexe Nationale

APPROVAL

Approved by the Director General of ASRO on

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Edition 1

## Forward

This standard represents the National Annex NA which defines the conditions of application on the territory of Romania of the standard SR EN 1993-1-6:2007 *Eurocode 3: Design of steel structures. Part 1-6: Strength and Stability of Shell Structures*, together with the corrigendum SR EN 1993-1-6:2007/AC:2009.

This National Annex has been prepared within the Technical Committee ASRO/CT 343 *Basis of the Design and Structural Eurocodes*, the secretariat of which is held by ASRO.

This National Annex:

- provides Nationally Determined Parameters (NDP) for the following clauses of the European Standard EN 1993-1-6:2007 and corrigendum EN 1993-1-6:2007/AC:2009, for which the national choice is allowed, namely:

- 3.1 (4)
- 4.1.4 (3)
- 5.2.4 (1)
- 6.3 (5)
- 7.3.1 (1)
- 7.3.2 (1)
- 8.4.2 (3)
- 8.4.3 (2)
- 8.4.3 (4)
- 8.4.4 (4)
- 8.4.5 (1)
- 8.5.2 (2)
- 8.5.2 (4)
- 8.7.2 (7)
- 8.7.2 (16)
- 8.7.2 (18) (2 ori)
- 9.2.1 (2)P

- establishes the conditions of application of normative annexes A, B, C and D from Romanian standard SR EN 1993-1-6:2007 and corrigendum SR EN 1993-1-6:2007/AC:2009.

SR EN 1993-1-6:2007/NA:2011 and SR EN 1993-1-6:2007 should be used together with the provisions of Eurocodes EN 1990 to EN 1999, after their national endorsement and implementation, together with their national annexes.

The names and the numbers of the clauses in the national annex are identical with those of the Romanian standard SR EN 1993-1-6:2007 and corrigendum SR EN 1993-1-6:2007/AC:2009.

This National Annex belongs to the patrimoine of ASRO/CT 343, *Basis of the Design and Structural Eurocodes*.

## I. Nationally Determined Parameters for SR EN 1993-1-6:2007 and corrigendum SR EN 1993-1-6:2007/AC:2009

### Section 3: Materials and geometry

#### 3.1 Material properties

(4) The material properties apply to temperatures not exceeding 150°C.

**NOTE** – The national annex may give information about material properties at temperatures exceeding 150°C.

For material properties at temperatures exceeding 150°C, the values given in SR EN 1993-1-2, Table 3.1 are used.

### Section 4: Ultimate limit states in steel shells

#### 4.1 Ultimate limit states to be considered

##### 4.1.4 LS4: Fatigue

(3) All variable actions that will be applied with more than  $N_f$  cycles in the design life time of the structure according to the relevant action spectrum in EN 1991 in accordance with the appropriate application part of EN 1993-3 or EN 1993-4, should be accounted for when checking LS4.

**NOTE** – The National Annex may choose the value of  $N_f$ . The value  $N_f = 10\,000$  is recommended.

The recommended value  $N_f = 10\,000$  is used.

### Section 5: Stress resultants and stresses in shells

#### 5.2 Modelling of the shell for analysis

##### 5.2.4 Stress resultants and stresses

(1) Provided that the radius to thickness ratio is greater than  $(r/t)_{\min}$ , the curvature of the shell may be ignored when calculating the stress resultants from the stresses in the shell wall.

**NOTA** – The National Annex may choose the value of  $(r/t)_{\min}$ . The value  $(r/t)_{\min} = 25$  is recommended.

The recommended value  $(r/t)_{\min} = 25$  is used.

### Section 6: Plastic limit state (LS1)

#### 6.3 Design by global numerical MNA or GMNA analysis

(5) Where a GMNA analysis is used, if the analysis predicts a maximum load followed by a descending path, the maximum value should be used to determine the load ratio  $r_{R,GMNA}$ . Where a GMNA analysis does not predict a maximum load, but produces a progressively rising action-displacement relationship without strain hardening of the material, the load ratio  $r_{R,GMNA}$  should be taken as no larger than the value at which the maximum von Mises equivalent plastic strain in the structure attains the value  $\varepsilon_{mps} = n_{mps} (f_{yd} / E)$ .

**NOTE** – The National Annex may choose the value of  $n_{mps}$ . The value  $n_{mps} = 50$  is recommended.

The recommended value  $n_{mps} = 50$  is used.

## Section 7: Cyclic plasticity limit state (LS2)

### 7.3 Design by global numerical MNA or GMNA analysis

#### 7.3.1 Design values of total accumulated plastic strain

(1) Where a materially nonlinear global numerical analysis (MNA or GMNA) is used, the shell should be subject to the design values of the varying and permanent actions detailed in 7.1.

NOTE 1 – It is usual to use an MNA analysis for this purpose.

NOTE 2 – The National Annex may give recommendations for a more refined analysis.

The use of an analysis MNA or GMNA is recommended.

#### 7.3.2 Total accumulated plastic strain limitation

(1) Unless a more sophisticated low cycle fatigue assessment is undertaken, the design value of the total accumulated von Mises equivalent plastic strain  $\varepsilon_{p,eq,Ed}$  should satisfy the condition:

$$\varepsilon_{p,eq,Ed} \leq n_{p,eq} (f_{yd} / E) \quad \dots (7.5)$$

NOTE – The National Annex may choose the value of  $n_{p,eq}$ . The value  $n_{p,eq} = 25$  is recommended.

The recommended value  $n_{p,eq} = 25$  is used.

## Section 8: Buckling limit state (LS3)

### 8.4 Buckling-relevant geometrical tolerances

#### 8.4.2 Out-of-roundness tolerance

(3) The out-of-roundness parameter  $U_r$  should satisfy the condition:

$$U_r \leq U_{r,max} \quad \dots (8.2)$$

where:

$U_{r,max}$  is the out-of-roundness tolerance parameter for the relevant fabrication tolerance quality class.

NOTE – Values for the out-of-roundness tolerance parameter  $U_{r,max}$  may be obtained from the National Annex. The recommended values are given in Table 8.1.

Table 8.1 – Recommended values for out-of-roundness tolerance parameter  $U_{r,max}$

	Diameter range	$d [m] \leq 0,50m$	$0,50m < d [m] < 1,25m$	$1,25m \leq d [m]$
Fabrication tolerance quality class	Description	Valoarea recomandată pentru $U_{r,max}$		
Class A	Excellent	0,014	$0,007 + 0,0093(1,25-d)$	0,007
Class B	High	0,020	$0,010 + 0,0133(1,25-d)$	0,010
Class C	Normal	0,030	$0,015 + 0,0200(1,25-d)$	0,015

Recommended values from Table 8.1 are used.

#### 8.4.3 Accidental eccentricity tolerance

(2) The accidental eccentricity  $e_a$  should be less than the maximum permitted accidental eccentricity  $e_{a,max}$ , for the relevant fabrication tolerance quality class.

NOTE – Values for the maximum permitted accidental eccentricity  $e_{a,max}$  may be obtained from the National Annex. The recommended values are given in Table 8.2.

**Table 8.2 – Recommended values for maximum permitted accidental eccentricities**

Fabrication tolerance quality class	Description	Recommended values for maximum permitted accidental eccentricity $e_{a,max}$
Class A	Excellent	2 mm
Class B	High	3 mm
Class C	Normal	4 mm

Recommended values from Table 8.2 are used.

(4) The accidental eccentricity parameter  $U_e$  should satisfy the condition:

$$U_e \leq U_{e,max} \quad (8.5)$$

where:

$U_{e,max}$  is the accidental eccentricity tolerance parameter for the relevant fabrication tolerance quality class.

**NOTE 1 – Values for the accidental eccentricity tolerance parameter  $U_{e,max}$  may be obtained from the National Annex. The recommended values are given in Table 8.3.**

**Table 8.3 – Recommended values for accidental eccentricity tolerances**

Fabrication tolerance quality class	Description	Recommended value of $U_{e,max}$
Class A	Excellent	0,14
Class B	High	0,20
Class C	Normal	0,30

Recommended values from Table 8.3 are used.

#### 8.4.4 Dimple tolerances

(4) The value of the dimple parameters  $U_{0x}$ ,  $U_{0\theta}$ ,  $U_{0w}$  should satisfy the conditions:

$$U_{0x} \leq U_{0,max} \quad U_{0\theta} \leq U_{0,max} \quad U_{0w} \leq U_{0,max} \quad \dots (8.10)$$

where:

$U_{0,max}$  is the dimple tolerance parameter for the relevant fabrication tolerance quality class.

**NOTE 1 – Values for the dimple tolerance parameter  $U_{0,max}$  may be obtained from the National Annex. The recommended values are given in Table 8.4.**

**Table 8.4 – Recommended values for dimple tolerance parameter  $U_{0,max}$**

Fabrication tolerance quality class	Description	Recommended value of $U_{0,max}$
Class A	Excellent	0,006
Class B	High	0,010
Class C	Normal	0,016

Recommended values from Table 8.4 are used.

#### 8.4.5 Interface flatness tolerance

(1) Where another structure continuously supports a shell (such as a foundation), its deviation from flatness at the interface should not include a local slope in the circumferential direction greater than  $\beta_\theta$ .

**NOTE – The National Annex may choose the value of  $\beta_\theta$ . The value  $\beta_\theta = 0,1\% = 0,001$  radians is recommended.**

The recommended value  $\beta_\theta = 0,1\% = 0,001$  radians is used.

## 8.5 Stress design

### 8.5.2 Design resistance (buckling strength)

(2) The partial factor for resistance to buckling  $\gamma_{M1}$  should be taken from the relevant application standard.

**NOTE** – The value of the partial factor  $\gamma_{M1}$  may be defined in the National Annex. Where no application standard exists for the form of construction involved, or the application standard does not define the relevant values of  $\gamma_{M1}$ , it is recommended that the value of  $\gamma_{M1}$  should not be taken as smaller than  $\gamma_{M1} = 1,1$ .

The recommended value  $\gamma_{M1} = 1,1$  is used.

(4) The buckling reduction factors  $\chi$ ,  $\chi_0$  și  $\chi_\tau$  should be determined as a function of the relative slenderness of the shell  $\bar{\lambda}$  from:

$$\chi = 1 \quad \text{when} \quad \bar{\lambda} \leq \bar{\lambda}_0 \quad \dots (8.13)$$

$$\chi = 1 - \beta \left( \frac{\bar{\lambda} - \bar{\lambda}_0}{\bar{\lambda}_p - \bar{\lambda}_0} \right)^\eta \quad \text{when} \quad \bar{\lambda}_0 < \bar{\lambda} < \bar{\lambda}_p \quad \dots (8.14)$$

$$\chi = \frac{\alpha}{\bar{\lambda}^2} \quad \text{when} \quad \bar{\lambda}_p \leq \bar{\lambda} \quad \dots (8.15)$$

where:

$\alpha$	is	the elastic imperfection reduction factor;
$\beta$	is	the plastic range factor;
$\eta$	is	the interaction exponent;
$\lambda_0$	is	the squash limit relative slenderness.

**NOTE 1** - The values of these parameters should be taken from Annex D. Where Annex D does not define the values of these parameters, they may be given by the National Annex.

Recommended values from Annex D are used.

## 8.7 Design by global numerical analysis using GMNIA analysis

### 8.7.2 Design value of resistance

(7) The largest tolerable deformation should be assessed relative to the conditions of the individual structure. If no other value is available, the largest tolerable deformation may be deemed to have been reached when the greatest local rotation of the shell surface (slope of the surface relative to its original geometry) attains the value  $\beta$ .

**NOTE** – The National Annex may choose the value of  $\beta$ . The value  $\beta = 0,1$  radians is recommended.

The recommended value  $\beta = 0,1$  radians is used.

(16) Modification of the adopted mode of geometric imperfections to include realistic structural details (such as axisymmetric weld depressions) should be explored.

**NOTE** - The National Annex may define additional requirements for the assessment of appropriate patterns of imperfections.

Additional requirements for the assessment of appropriate patterns of imperfections are not defined.

(18) The amplitude of the adopted equivalent geometric imperfection form should be taken as dependent on the fabrication tolerance quality class. The maximum deviation of the

geometry of the equivalent imperfection from the perfect shape  $\Delta w_{0,eq}$  should be the larger of  $\Delta w_{0,eq,1}$  and  $\Delta w_{0,eq,2}$ , where:

$$\Delta w_{0,eq,1} = \ell_g U_{n1} \quad \dots (8.29)$$

$$\Delta w_{0,eq,2} = \eta_i t U_{n2} \quad \dots (8.30)$$

where:

$\ell_g$  is all relevant gauge lengths according to 8.4.4 (2);

$t$  is the local shell wall thickness;

$\eta_i$  is a multiplier to achieve an appropriate tolerance level;

$U_{n1}$  and  $U_{n2}$  are the dimple imperfection amplitude parameters for the relevant fabrication tolerance quality class.

**NOTE 1** - The National Annex may choose the value of  $\eta_i$ . The value  $\eta_i = 25$  is recommended.

The recommended value  $\eta_i = 25$  is used.

**NOTE 2** – Values for the dimple tolerance parameter  $U_{n1}$  and  $U_{n2}$  may be obtained from the National Annex. The recommended values are given in Table 8.5.

**Table 8.5 - Recommended values for dimple imperfection amplitude parameters  $U_{n1}$  and  $U_{n2}$**

Fabrication tolerance quality class	Description	Recommended value of $U_{n1}$	Recommended value of $U_{n2}$
Class A	Excellent	0,010	0,010
Class B	High	0,016	0,016
Class C	Normal	0,025	0,025

Recommended values from Table 8.5 are used.

## Section 9: Fatigue limit state (LS4)

### 9.2 Stress design

#### 9.2.1 General

(2)P The partial factor for resistance to fatigue  $\gamma_{Mf}$  shall be taken from the relevant application standard.

**NOTE** - The value of the partial factor  $\gamma_{Mf}$  may be defined in the National Annex. Where no application standard exists for the form of construction involved, or the application standard does not define the relevant values of  $\gamma_{Mf}$ , the value of  $\gamma_{Mf}$  should be taken from EN 1993-1-9. It is recommended that the value of  $\gamma_{Mf}$  should not be taken as smaller than  $\gamma_{Mf} = 1,1$ .

The recommended value  $\gamma_{Mf} = 1,1$  is used.

## **II. Conditions of application of SR EN 1993-1-6:2007 Annexes and corrigendum SR EN 1993-1-6:2007/AC:2009**

### **Annex A (normative): Membrane theory stresses in shells**

Annex A remains informative for the purpose of SR EN 1993-1-6:2007, corrigendum SR EN 1993-1-6:2007/AC:2009 and National Annex.

### **Annex B (normative): Additional expressions for plastic collapse resistances**

Annex B remains informative for the purpose of SR EN 1993-1-6:2007, corrigendum SR EN 1993-1-6:2007/AC:2009 and National Annex.

### **Annex C (normative): Expressions for linear elastic membrane and bending stresses**

Annex C remains informative for the purpose of SR EN 1993-1-6:2007, corrigendum SR EN 1993-1-6:2007/AC:2009 and National Annex.

### **Annex D (normative): Expressions for buckling stress design**

Annex D remains informative for the purpose of SR EN 1993-1-6:2007, corrigendum SR EN 1993-1-6:2007/AC:2009 and National Annex.



Romanian standards include necessary provisions for contracting. Provisions are not limitative. Standard users are responsible for its enforcement.

It is important that users of Romanian standards ensure they have the latest edition and all changes.

Informations on Romanian standards are published in the *Romanian Standards Catalogue* and in the *Standards Bulletin* retrieving themselves also in the information and documentation *Infostandard*, in electronic format, which is actualized weekly.